

A Bi-axial Failure Envelop for Concrete under High Strain Rate Conditions

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ABSTRACT

Since concrete is an anisotropic material, dynamic bi-axial behavior of concrete should be considered to describe concrete structure subjected blast or impact loading. Although several dynamic bi-axial models for concrete have been proposed to describe the high strain rate bi-axial behavior of concrete, it is difficult to reflect the coupled effect of strain rate and lateral pressure reasonably. To solve this problem, this paper introduces a bi-axial model for concrete under high strain rate conditions by combining HJC (Holmquist Johnson Cook) model and bi-axial model proposed by Yan. The Proposed model takes into account the strain rate effect with HJC model and the confining pressure effect with Yan model independently.

1. INTRODUCTION

With increasing need to ensure safety of concrete structures from explosions, which recently have occurred frequently, as well as against the risk of natural hazards such as tornado, various studies ranging from identification of dynamic material properties to analysis of structural responses under blast and impact loadings have been conducted. Concrete structures subjected to blast or impact loading present significantly different structural behaviour from that observed under static loading conditions. In this context, numerous studies, from the Marais experiment (Marais 2004), which identified material properties of various materials with the SHPB (Split Hopkinson Pressure Bar) test, to research by Georgin et al (Georgin 2003), who carried out a numerical analysis for structures under impact loading, have been conducted. The obtained research results have been used in developing design codes such as the CEB-FIP model code (CEB-FIB 1990) and also have been implemented into many commercialized programs including LS-DYNA (Hallquist 2007) and ABAQUS (Hibbitt 2001) to be used in tracing the nonlinear response of structures subjected to blast and impact loadings. Unfortunately, most research to date has been conducted in relation to uniaxial high strain rate condition, partly because of the difficulties involved in multi-axial dynamic experiments for concrete. However it is unable to reflect the coupled effect of strain rate and lateral pressure reasonably. To solve the problem, this paper introduces a bi-axial model for concrete under high strain rate conditions by combining HJC (Holmquist Johnson Cook) model (Holmquist 1993) and bi-axial model proposed

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by Yan (Yan 2007). The Proposed model takes into account the strain rate effect with HJC model and the confining pressure effect with Yan model independently. Furthermore, High strain rate biaxial stress-strain curves for compression-compression region determined by proposed model are compared with experimental results, HJC (Holmquist Johnson Cook) model and K&C (Karagozian & Case) model (Schwer 2005) to verify accuracy of the proposed model.

2. High strain rate bi-axial model for concrete

2.1 HJC (Holmquist Johnson Cook) model

HJC model (Holmquist 1993) is a computational constitutive model for concrete subjected to large strains, high strain rate, and high pressure. Equation for HJC model is expressed in Eq. (1).

$$\sigma^* = [A(1 - D_c) + BP^{*N}](1 + C \ln(\dot{\varepsilon}^*)) \quad (1)$$

Where, A is the normalized cohesive strength, B is the normalized pressure hardening coefficient, N is the pressure hardening exponent, C is the strain rate coefficient, D is damage, $\sigma^* = \sigma/f'_c$ is normalized stress (where f'_c is quasi-static uniaxial compressive strength), $P^* = P/f'_c$ is normalized pressure (where P is the actual pressure), and $\dot{\varepsilon}^* = \dot{\varepsilon}/\dot{\varepsilon}_0$ is the dimensionless strain rate (where $\dot{\varepsilon}$ is the actual strain rate and $\dot{\varepsilon}_0$ is the reference strain rate). As shown in Fig. 1, when literature value is used for material constant A, B, N, and C, HJC model (strain rate: 272/s, 357/s, 460/s) gives unsatisfactory agreement with experimental results. In this reason, this paper uses own value for material constants determined on the basis of SHPB (Split-Hopkinson Pressure Bar) test and triaxial compression test. SHPB (Split-Hopkinson Pressure Bar) test has been performed for strain rate 100/s – 600/s and triaxial compression test has been performed for confinement pressure 4Mpa – 10Mpa. As shown in Fig. 2, model construction program has been used for finding material constants A, B, N, C on the basis of experimental results. As a results, Material constants which show good agreement with experimental stress-strain curve on the basis of least square method is A=0.698, B=2, N=1.3, C=0.009.

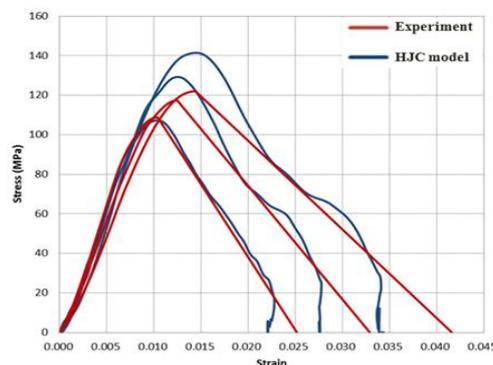


Fig. 1 Comparison between HJC model using literature value for material constant and experimental results

As shown in Fig. 3, HJC model shows satisfactory agreement with experimental results with own value for material constants. Since HJC model has been used to account for strain rate effects in this paper, uniaxial compression strength calculated by HJC model using $P^* = \frac{1}{3} \frac{\sigma}{f'_c}$ in order to eliminate confinement effect is used in high strain rate bi-axial model suggested in this paper.

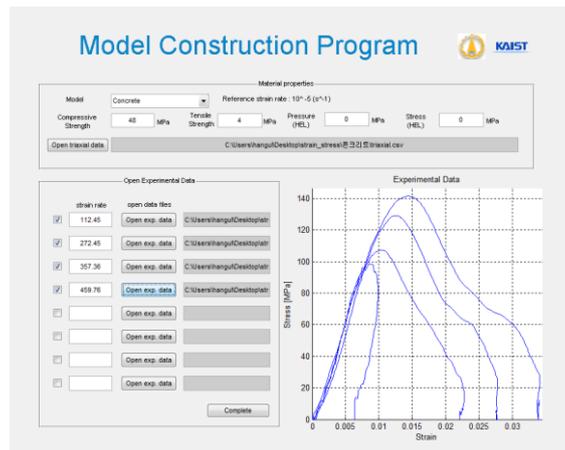


Fig. 2 Model construction program

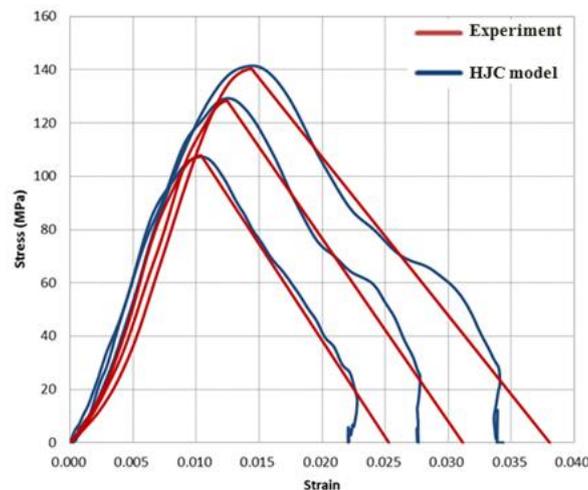


Fig. 3 Comparison between HJC model using own value for material constant and experimental results

2.2 Biaxial model proposed by Yan

Biaxial model proposed by Yan[12] takes account for both the effect of strain rate and the effect of stress ratio on the ultimate strength of concrete in biaxial stress states. Yan model is suggested as follows

$$R(\alpha) = P_1 + P_2 \log \left(\frac{\dot{\varepsilon}}{\dot{\varepsilon}_s} \right) + \frac{P_3}{(1+\alpha)^2} + \frac{P_4 \alpha}{(1+\alpha)^2} \quad (2)$$

Where P_1 , P_2 , P_3 and P_4 represent the parameters associated with material properties, $\alpha = \frac{\sigma_2}{\sigma_1}$ is the stress ratio, $\dot{\varepsilon}$ is the strain rate and $\dot{\varepsilon}_s$ is the quasi-static strain rate, its value being selected as $10^{-5}/s$ in this paper. It is noted that Yan model takes into account the strain-rate effect and the confining pressure effect independently and thus it is unable to reflect the coupled effect of strain rate and lateral pressure reasonably (Yan 2007). Therefore, Yan model is modified to eliminate strain rate effect in it by dividing $R(\alpha)$ with $R(0)$ in this paper. Modified Yan model has been used for high strain rate bi-axial model suggested in this paper to account for lateral confinement effect of concrete.

2.3 Proposal of high strain rate bi-axial model for concrete

High strain rate bi-axial model for concrete is proposed by combining uniaxial strength calculated by HJC model using own material constants and failure envelop of modified Yan model. The Proposed model takes into account the strain rate effect with HJC model and the confining pressure effect with Yan model independently. Failure envelop consists of compression-compression region using proposed model in Eq. (3), tension-tension region using CEB-FIP code (CEB-FIB 1990) and compression-tension region using linear interpolation.

$$f(\alpha) = \frac{R(\alpha)}{R(0)} \cdot \left[A + B \left(\frac{1}{3} \frac{\sigma}{f_c} \right)^N \right] [1 + C \ln(\dot{\varepsilon})] \quad (3)$$

The Proposed bi-axial failure envelop is visualized in Fig. 4. Strength of concrete, which takes account for both strain rate effect and confinement effect, is estimated with proposed failure envelop. Stress-strain curve is estimated on the basis of strength calculated by proposed failure envelop.

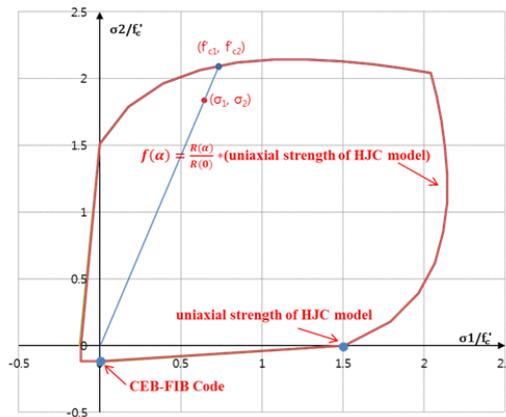
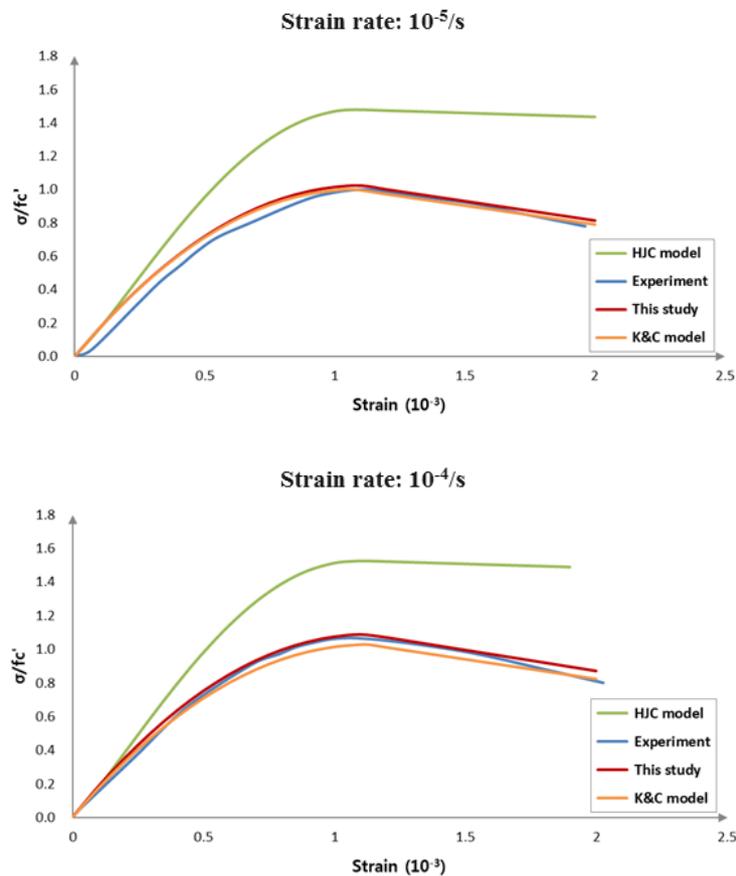


Fig. 4 Proposed bi-axial high strain rate model for concrete

3. Comparison with experimental results and preceding models

Stress-strain curve for compression part suggested by proposed bi-axial model has been compared with experimental results (Yan 2007), HJC model (Holmquist 1993) using literature value for material constants and K&C model to testify the accuracy of the proposed bi-axial model. The strain rate varied from $10^{-5}/s$ to $10^{-2}/s$ and lateral pressure was maintained at a fixed proportion to the axial load, with the stress ratios of 0:1 and 1:1 respectively. Comparison is needed at higher strain rates in order to ensure the reliability of the proposed bi-axial model, but comparison at low strain rate was performed due to lack of experimental data. Comparison results are shown in Fig. 5 and 6. In the case of stress ratio of 0:1, strain rate effect is only considered in stress-strain curve because it is in uniaxial compression condition. As shown in Fig. 5, K&C model and proposed model show good agreement with experimental results, whereas HJC model overestimates stress conditions.



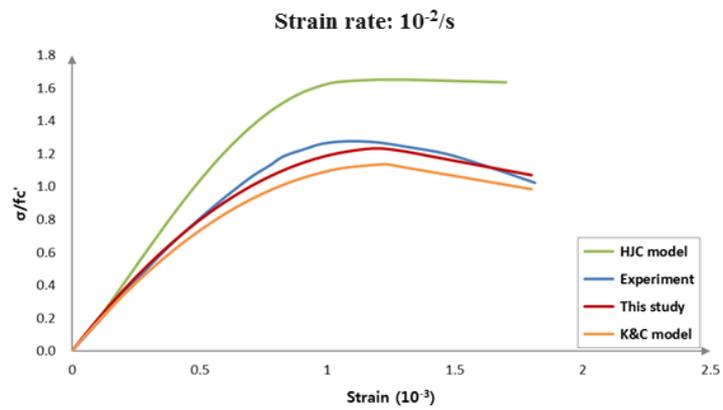
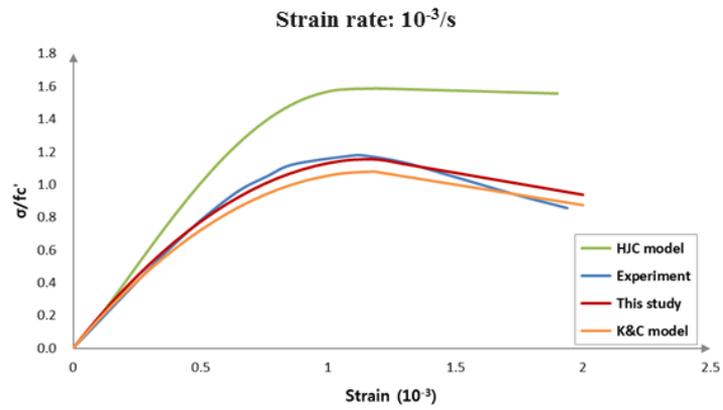
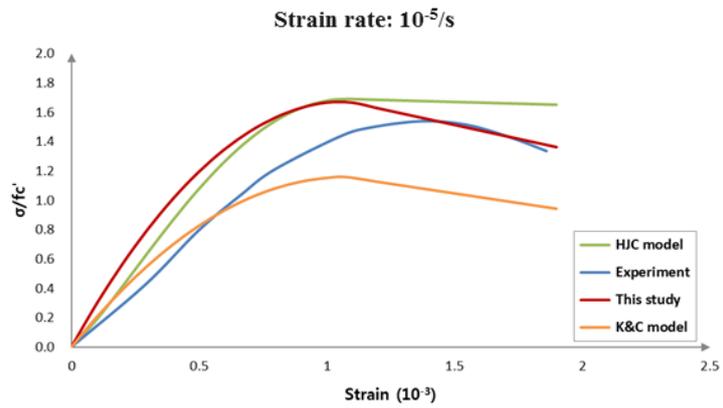


Fig. 5 Stress-strain curve for stress ratio of 0:1



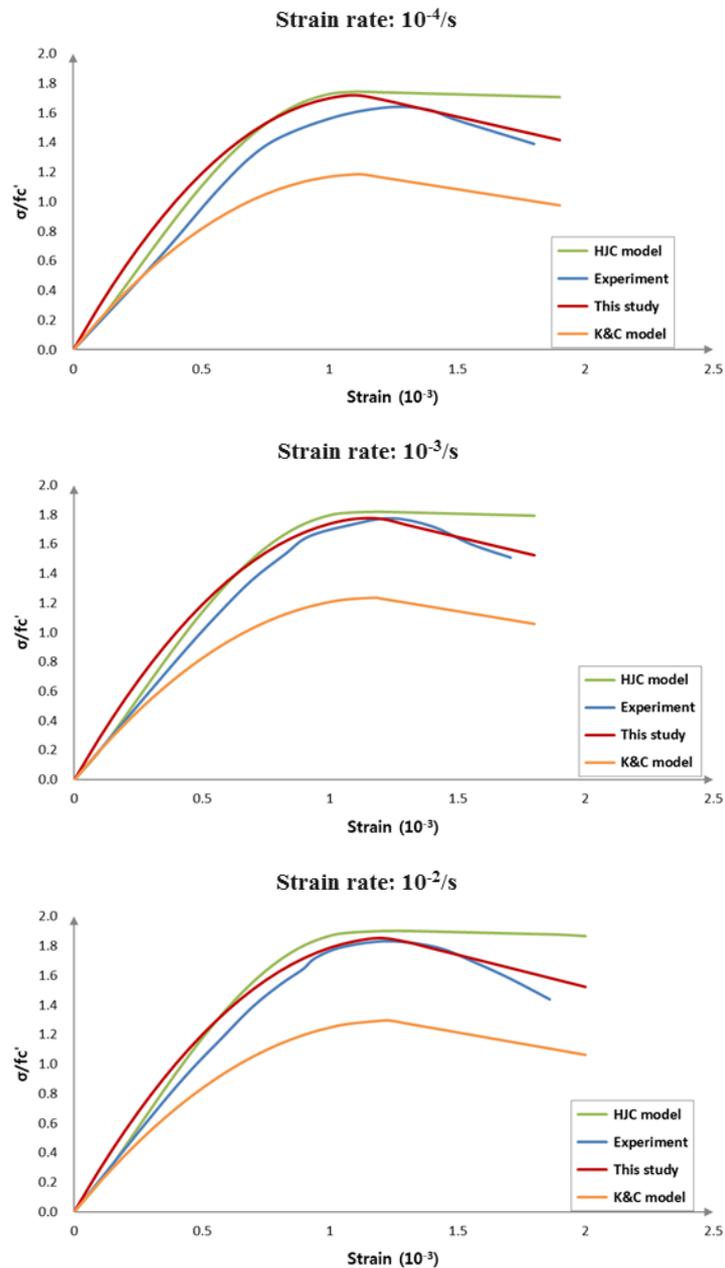


Fig. 6 Stress-strain curve for stress ratio of 1:1

In the case of stress ratio of 1:1, HJC model overestimates stress conditions and K&C model underestimates stress conditions. As a result, bi-axial model proposed in this paper can be used to describe compressive bi-axial behaviour of concrete under high strain rate conditions.

4. CONCLUSIONS

This paper introduces a bi-axial model for concrete under high strain rate conditions by

combining HJC (Holmquist Johnson Cook) model and bi-axial model proposed by Yan. The Proposed model takes into account the strain rate effect with HJC model and the confining pressure effect with Yan model independently. Furthermore, stress-strain curve for compression part suggested by proposed bi-axial model has been compared with experimental results[12], HJC model[10] using literature value for material constants and K&C model[11] to testify the accuracy of the proposed bi-axial model. As a result, bi-axial model proposed in this paper can be used to describe compressive bi-axial behaviour of concrete under high strain rate conditions.

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